

Test Report No. 616221-01 Test Report Date: December 2022

MASHTL-4 CRASH TESTING OF BICYCLE RAILING ON A CONSTANT SLOPE PARAPET

by

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Sponsored by **Illinois Department of Transportation**

TEXAS A&M TRANSPORTATION INSTITUTE PROVING GROUND

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The results reported herein apply only to the article tested. The full-scale crash test was performed according to TTI Proving Ground quality procedures and American Association of State Highway and Transportation Officials (AASHTO) Manual for Assessing Safety Hardware, Second Edition (*MASH*) guidelines and standards.

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*SI is the symbol for the International System of Units

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Chapter 1. INTRODUCTION

1.1. PROBLEM

The purpose of this project is to evaluate and test a combination barrier system that consists of a bicycle railing mounted on top of a standard Illinois Department of Transportation (IDOT) concrete parapet. The total height of the system is 54 inches, including a 15-inch tall bicycle railing mounted on top of a 39-inch tall constant slope parapet. The testing was performed in accordance with Test Level 4 (TL-4) criteria of the American Association of State Highway and Transportation Official's (AASHTO) Manual for Assessing Safety Hardware (*MASH*) (1).

1.2. BACKGROUND

IDOT has a significant number of bridges that accommodate bicyclists throughout the state. The department utilizes a railing height of 54 inches as recommended in the early editions of the AASHTO Load and Resistance Factor Design (LRFD) Bridge Design Specifications. *MASH* TL-4 compliant barrier systems are recommended on IDOT bridges whenever possible. Therefore, IDOT wanted to develop and test a 15-inch tall railing mounted on top of a 39-inch tall standard IDOT parapet to accommodate bicyclist and traffic safety.

1.3. OBJECTIVES

The purpose of the testing reported herein was to assess the performance of the railing mounted on top of a standard IDOT constant slope parapet according to the safety-performance evaluation guideline included in AASHTO *MASH* for TL-4 longitudinal barriers.

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Chapter 2. DESIGN AND ANALYSIS

The TTI research team evaluated the railing design details and configuration presented in [Figure 2.1](#page-13-1) according to AASHTO LRFD Section 13 and concluded that the system is structurally adequate for *MASH* TL-4 impact conditions. Additional recommendations were provided to improve the continuity of the rail at joint locations to reduce the possibility of vehicle snagging.

Figure 2.1 Combination Barrier System

Chapter 3. SYSTEM DETAILS

 3.1. TEST ARTICLE AND INSTALLATION DETAILS

The installation consisted of a 122 foot - 2 inch long concrete parapet with a double square tube rail mounted on top. The concrete parapet was 39 inches tall, 17 inches wide at the bottom and then sloped towards the field side on the traffic side for a width of 9½ inches at the top. It was anchored into a concrete deck 8 inches thick and 48 inches wide. There was a 2-inch relief joint in the concrete parapet 31 feet and 1 inch from the upstream end of the installation. The double rail and posts mounted on top of the parapet was constructed of HSS 3" x 3" x $\frac{1}{4}$ ", and was 54 inches from the deck to the top of the rail. The posts were anchored to the parapet with two threaded rods on the traffic side of the base plates.

Figure 3.1 presents the overall information on the Bicycle Railing on a Constant Slope Parapet, and Figure 3.2 provides photographs of the installation. Appendix A provides further details on the Bicycle Railing on a Constant Slope Parapet. Drawings were provided by the Texas A&M Transportation Institute (TTI) Proving Ground, and construction was performed by MBC Management and supervised by TTI Proving Ground personnel.

3.2. DESIGN MODIFICATIONS DURING TESTS

No modifications were made to the installation during the testing phase.

3.3. MATERIAL SPECIFICATIONS

Appendix B provides material certification documents for the materials used to install/construct the Bicycle Railing on a Constant Slope Parapet. [Table 3.1](#page-14-4) shows the average compressive strengths of the concrete on November 8, 2021, the day of the test:

Figure 3.1 Details of Bicycle Railing on a Constant Slope Parapet.

Figure 3.2 Bicycle Railing on a Constant Slope Parapet prior to Testing.

Chapter 4. TEST REQUIREMENTS AND EVALUATION CRITERIA

 4.1. CRASH TEST PERFORMED/MATRIX

Table 4.1 shows the test conditions and evaluation criteria for *MASH* TL-4 for longitudinal barriers.

It should be noted that MASH TL-4 criteria also requires testing with a small passenger car (1100C vehicle) and pickup truck (2270P vehicle). * However, MASH Test 4-10 and MASH Test 4-11 are not critical due to successfully performed tests on similar systems in the past (2- 4). Furthermore, the small car and pickup truck will not impart a greater load into the combination barrier system in comparison to the single unit truck of MASH Test 4-12. Thus, only MASH Test 4-12 was performed under this project.

The target critical impact point (CIP) for the test was determined using the information provided in *MASH* Section 2.2.1 and Section 2.3.2. Figure 4.1 shows the target CIP for *MASH* Test 4-12 on the Bicycle Railing on a Constant Slope Parapet.

Figure 4.1 Target CIP for *MASH* **TL-4 Tests on Bicycle Railing on a Constant Slope Parapet.**

The crash tests and data analysis procedures were in accordance with guidelines presented in *MASH*. Chapter 4 presents brief descriptions of these procedures.

 4.2. EVALUATION CRITERIA

The appropriate safety evaluation criteria from Tables 2-2 and 5-1 of *MASH* were used to evaluate the crash test reported herein. Table 4-1 lists the test conditions and evaluation criteria required for *MASH* TL-4, and Table 4.2 provides detailed information on the evaluation criteria.

Table 4.2 Evaluation Criteria Required for *MASH* **Testing.**

Chapter 5. TEST CONDITIONS

5.1. TEST FACILITY

The full-scale crash test reported herein was performed at the TTI Proving Ground, an International Standards Organization (ISO)/International Electrotechnical Commission (IEC) 17025-accredited laboratory with American Association for Laboratory Accreditation (A2LA) Mechanical Testing Certificate 2821.01. The full-scale crash test(s) was/were performed according to TTI Proving Ground quality procedures, as well as *MASH* guidelines and standards.

The test facilities of the TTI Proving Ground are located on The Texas A&M University System RELLIS Campus, which consists of a 2000-acre complex of research and training facilities situated 10 mi northwest of the flagship campus of Texas A&M University. The site, formerly a United States Army Air Corps base, has large expanses of concrete runways and parking aprons well suited for experimental research and testing in the areas of vehicle performance and handling, vehicle-roadway interaction, highway pavement durability and efficacy, and roadside safety hardware and perimeter protective device evaluation. The sites selected for construction and testing are along the edge of an out-of-service apron/runway. The apron/runways consists of an unreinforced jointed-concrete pavement in 12.5-ft \times 15-ft blocks nominally 6 inches deep. The aprons were built in 1942, and the joints have some displacement but are otherwise flat and level.

5.2. VEHICLE TOW AND GUIDANCE SYSTEM

The vehicle was towed into the test installation using a steel cable guidance and reverse tow system. A steel cable for guiding the test vehicle was tensioned along the path, anchored at each end, and threaded through an attachment to the front wheel of the test vehicle. An additional steel cable was connected to the test vehicle, passed around a pulley near the impact point and through a pulley on the tow vehicle, and then anchored to the ground such that the tow vehicle moved away from the test site. A 2:1 speed ratio between the test and tow vehicle existed with this system. Just prior to impact with the installation, the test vehicle was released and ran unrestrained. The vehicle remained freewheeling (i.e., no steering or braking inputs) until it cleared the immediate area of the test site.

 DATA ACQUISITION SYSTEMS

 Vehicle Instrumentation and Data Processing

The test vehicle was instrumented with a self-contained onboard data acquisition system. The signal conditioning and acquisition system is a 16-channel Tiny Data Acquisition System (TDAS) Pro produced by Diversified Technical Systems Inc. The accelerometers, which measure the x, y, and z axis of vehicle acceleration, are strain gauge type with linear millivolt output proportional to acceleration. Angular rate sensors, measuring vehicle roll, pitch, and yaw rates, are ultra-small, solid-state units designed for crash test service. The TDAS Pro hardware and software conform to the latest SAE J211, Instrumentation for Impact Test. Each of the 16 channels is capable of providing precision amplification, scaling, and filtering based on

transducer specifications and calibrations. During the test, data are recorded from each channel at a rate of 10,000 samples per second with a resolution of one part in 65,536. Once data are recorded, internal batteries back these up inside the unit in case the primary battery cable is severed. Initial contact of the pressure switch on the vehicle bumper provides a time zero mark and initiates the recording process. After each test, the data are downloaded from the TDAS Pro unit into a laptop computer at the test site. The Test Risk Assessment Program (TRAP) software then processes the raw data to produce detailed reports of the test results.

Each of the TDAS Pro units is returned to the factory annually for complete recalibration and to ensure that all instrumentation used in the vehicle conforms to the specifications outlined by SAE J211. All accelerometers are calibrated annually by means of an ENDEVCO® 2901 precision primary vibration standard. This standard and its support instruments are checked annually and receive a National Institute of Standards Technology (NIST) traceable calibration. The rate transducers used in the data acquisition system receive calibration via a Genisco Rateof-Turn table. The subsystems of each data channel are also evaluated annually, using instruments with current NIST traceability, and the results are factored into the accuracy of the total data channel per SAE J211. Calibrations and evaluations are also made anytime data are suspect. Acceleration data are measured with an expanded uncertainty of ± 1.7 percent at a confidence factor of 95 percent $(k = 2)$.

TRAP uses the data from the TDAS Pro to compute the occupant/compartment impact velocities, time of occupant/compartment impact after vehicle impact, and highest 10-millisecond (ms) average ridedown acceleration. TRAP calculates change in vehicle velocity at the end of a given impulse period. In addition, maximum average accelerations over 50-ms intervals in each of the three directions are computed. For reporting purposes, the data from the vehicle-mounted accelerometers are filtered with an SAE Class 180-Hz low-pass digital filter, and acceleration versus time curves for the longitudinal, lateral, and vertical directions are plotted using TRAP.

TRAP uses the data from the yaw, pitch, and roll rate transducers to compute angular displacement in degrees at 0.0001-s intervals, and then plots yaw, pitch, and roll versus time. These displacements are in reference to the vehicle-fixed coordinate system with the initial position and orientation being initial impact. Rate of rotation data is measured with an expanded uncertainty of ± 0.7 percent at a confidence factor of 95 percent (k = 2).

5.3.2. Anthropomorphic Dummy Instrumentation

MASH does not recommend or require use of a dummy in the 10000S vehicle, and no dummy was placed in the vehicle.

5.3.3. Photographic Instrumentation Data Processing

Photographic coverage of the test included three digital high-speed cameras:

One overhead with a field of view perpendicular to the ground and directly over the impact point.

One placed upstream from the installation at an angle to have a field of view of the interaction of the rear of the vehicle with the installation.

A third placed with a field of view parallel to and aligned with the installation at the downstream end.

A flashbulb on the impacting vehicle was activated by a pressure-sensitive tape switch to indicate the instant of contact with the Bicycle Railing on a Constant Slope Parapet. The flashbulb was visible from each camera. The video files from these digital high-speed cameras were analyzed to observe phenomena occurring during the collision and to obtain time-event, displacement, and angular data. A digital camera recorded and documented conditions of the test vehicle and the installation before and after the test.

MASH TEST 4-12 (CRASH TEST NO. 616221-01)

6.1. TEST DESIGNATION AND ACTUAL IMPACT CONDITIONS

See [Table 6.1](#page-22-3) for details on *MASH* impact conditions for this test. [Figure 6.1](#page-22-2) depicts the target impact setup.

Figure 6.1 Bicycle Railing on a Constant Slope Parapet Test Vehicle Geometrics for Test 616221-01.

Table 6.1 Impact Conditions for *MASH* **Test 4-12 / Test 616221-01.**

6.2. WEATHER CONDITIONS

Table 6.2 Weather Conditions 616221-01.

6.3. TEST VEHICLE

[Figure 6.2](#page-23-2) shows the 2004 Freightliner M21G6 used for the crash test. [Table 6.3](#page-23-4) shows the vehicle measurements. Table C.1 in Appendix C.1 gives additional dimensions and information on the vehicle.

Figure 6.2 Test Vehicle before Test 616221-01.

Test Parameter	MASH	Allowed Tolerance	Actual Measured
Curb Weight (lbs)	13,200	$\pm 2,200$	14,760
Gross Static (lbs)	22,046	660	22,590
Wheelbase (inches)	240	\leq 240	207
Overall Length (inches)	394	$<$ 394	332.5
Cargo Bed Height (inches) ^a	49	± 2	51
CG of Ballast above Ground ^b (inches)	63	$+2$	64.8

Table 6.3 Vehicle Measurements 616221-01.

a– Without Ballast

b – See section 4.2.1.2 in *MASH* 2016 for recommended ballasting procedures

6.4. TEST DESCRIPTION

[Table 6.4](#page-24-2) lists events that occurred during Test No. 616221-01. Figures C.1 and C.2 in Appendix C.2 present sequential photographs during the test.

Table 6.4 Events during Test 616221-01.

6.5. DAMAGE TO TEST INSTALLATION

There was scuffing and gouging at the impact location, and spalling on the field side of the joint at posts 4 and 5, which exposed rebar on both sections of the parapet. There was also a major crack on the field side of the parapet at both posts 4 and 5, and the top rail was permanently deformed towards the field side.

[Figure 6.3](#page-25-1) shows the damage to the Bicycle Railing on a Constant Slope Parapet. [Table](#page-24-3) [6.5](#page-24-3) describes the damage to the Bicycle Railing on a Constant Slope Parapet.

^{*} Per *MASH*, "The working width is the maximum dynamic lateral position of any major part of the system or vehicle. These measurements are all relative to the pre-impact traffic face of the test article." In other words, working width is the total barrier width plus the maximum dynamic intrusion of any portion of the barrier or test vehicle past the field side edge of the barrier.

Figure 6.3 Bicycle Railing on a Constant Slope Parapet after Test 616221-01.

6.6. DAMAGE TO TEST VEHICLE

[Figure 6.4](#page-25-2) and [Figure 6.5](#page-26-1) shows the damage sustained by the vehicle. [Table 6.6](#page-26-2) provide details on the interior and exterior damage to the vehicle. Tables C.2 and C.3 in Appendix C.1 provide exterior crush and occupant compartment measurements.

Figure 6.4 Test Vehicle after Test 616221-01.

Figure 6.5 Interior of Test Vehicle after Test 616221-01.

Table 6.6 Damage to Vehicle 616221-01.

The front bumper, hood, left headlight, left front tire and rim, left spring assembly, left u-bolt, left door, left side step, left side center floor pan, left side lower edge of box, and left rear outer tire and rim were damaged.

6.7. OCCUPANT RISK FACTORS

Data from the accelerometers were digitized for evaluation of occupant risk, and the results are shown in [Table 6.7](#page-27-0) Figure C.3 in Appendix C.3 shows the vehicle angular displacements, and Figures C.4 through C.6 in Appendix C.4 show acceleration versus time traces.

Test Parameter	MASH	Measured	Time	
OIV , Longitudinal (ft/s)	≤ 40.0	5.8	0.1890 s on left side of interior	
OIV , Lateral (ft/s)	≤ 40.0	10.7	0.1890 s on left side of interior	
Ridedown, Longitudinal (g)	≤ 20.49	5.9	$0.2640 - 0.2740$ s	
Ridedown, Lateral (g)	≤ 20.49	12.8	$0.2663 - 0.2763$ s	
THIV (m/s)	N/A	3.8	0.1829 s on left side of interior	
ASI	N/A	0.6°	$0.2686 - 0.3186$ s	
50ms MA Longitudinal (g)	N/A	-2.0	$0.0512 - 0.1012$ s	
50ms MA Lateral(g)	N/A	4.8	$0.2290 - 0.2790$ s	
50ms MA Vertical (g)	N/A	2.0	$0.3714 - 0.4214$ s	
Roll (deg)	\leq 75	28	0.7130 s	
Pitch (deg)	\leq 75	9	0.7249 s	
Yaw (deg)	N/A	23	0.9622 s	
Comments:				

Table 6.7 Occupant Risk Factors for Test 616221-01.

Chapter 7. SUMMARY AND CONCLUSIONS

7.1. ASSESSMENT OF TEST RESULTS

The crash test reported herein was performed in accordance with *MASH* Test 4-12, which involved one test, on the Bicycle Railing on a Constant Slope Parapet. Tables at the end of this section provide an assessment of the test based on the applicable safety evaluation criteria for *MASH* TL-4 longitudinal barriers.

7.2. CONCLUSIONS

Table 7.1 shows that the Bicycle Railing on a Constant Slope Parapet met the performance criteria for *MASH* 4-12 longitudinal barriers.

Table 7.1 Performance Evaluation Summary for *MASH* **Test 4-12 on Bicycle Railing on a Constant Slope Parapet.**

Evaluation Factors	Evaluation Criteria	Test No. 616221-01	
Structural Adequacy	A	S	
Occupant Risk	D	S	
	\mathbf{F}	N/A	
	G	S	
	H	N/A	
	I	N/A	
Test No.		MASH Test 4-12	
Pass/Fail		Pass	

Table 7.2 Assessment Summary for *MASH* **Test 4-12 on Bicycle Railing on a Constant Slope Parapet.**

Note: S = Satisfactory; N/A = Not Applicable.

REFERENCES

- 1. AASHTO. Manual for Assessing Safety Hardware (*MASH*), Second Edition. American Association of State Highway and Transportation Officials, Washington, DC, 2016.
- 2. W.F. Williams, R.P. Bligh, and W.L. Menges, Mash Test 3-11 of the TxDOT Single Slope Bridge Rail (Type SSTR) on Pan-Formed Bridge Deck. Report 9-1002-3. Texas A&M Transportation Institute, College Station, TX, 2011.
- 3. D. Whitesel, J. Jewell, and R. Meline, Compliance Crash Testing of the Type 60 Median Barrier, Test 140MASH3C16-04. Research Report FHWA/CA17-2654, Roadside Safety Research Group, California Department of Transportation, Sacramento, CA, May 2018.
- 4. Sheikh, N.M., Bligh, R. P., and Menges, W.L. (2009). "Development and Testing of a Concrete Barrier Design for Use in Front of Slope or on MSE Wall." Report 405160- 13-1, Texas A&M Transportation Institute, College Station, Texas.

APPENDIX A. DETAILS OF BICYCLE RAILING ON A CONSTANT SLOPE PARAPET

Q:\Accreditation-17025-2017\EIR-000 Project Files\616221-01 TL-4 Bicycle Railing on Constant Slope Parapet - Kiani\Drafting, 616221\616221 Drawing

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APPENDIX B. SUPPORTING CERTIFICATION DOCUMENTS

The following concrete mix designs are proposed for use at the above referenced project. The concrete compressive strength historical data submitted was derived from past mix designs produced by our ready-mix plants and analyzed in accordance with ACI 214 or may be by the laboratory trial mixture method.

Mix **Use** FN950200801 **Test Barrier**

To ensure the correct mix is delivered to your project, please order by the mix design code as indicated above.

The above mixes have been proportioned in accordance with the required sections in ACI 211 and/or your request. The above designs will meet or exceed the indicated specified strength when the concrete is tested in accordance with the applicable and current ASTM Standards and evaluated in accordance with the ACI 318 Building Code. Failure to test concrete specimens in accordance with specifications will void all warranties. Any additional products added to the concrete outside of the normal batching procedure will be at the liability of the purchaser.

Please notify Texcrete of approval of the proposed mix design prior to their use. Failure to notify us prior to the first placement shall constitute acceptance. To ensure that the proper mix designs are ordered, please send a copy of this letter, after approval, to the above referenced project to be used by the person ordering concrete for this job.

Texcrete would like to be included on the mailing list to receive test reports at will@texcrete.net. ASTM C94 and ACI 301 entitles the manufacturer to receive copies of the test reports when the strength of the concrete is used as the basis for acceptance. This will not happen automatically it will require a request on your part to the testing laboratory and owner. This information allows us to monitor your project as well as develop statistical histories for your future projects.

Please contact us if you have any questions or require any additional information.

Respectfully Submitted,

Will Squyres Director of Technical Services

Bryan ы уап
5222 Sandy Point Road
Bryan, Texas 77807
979-985-3636

Mix Properties

Texcrete has no knowledge or authority regarding where this mix is to be placed, therefore it is the responsibility of the project architect/engineer, and/or the contractor to ensure that the above mix design parameters of compressive strength, water/cementitious ratio, cement content, and air content are appropriate for the anticipated environmental conditions (i.e. ACI 318, ACI 301, and the local Building Codes).

The data enclosed represents the potential of this mix when sampled, cured, and tested per the appropriate and current ACI and ASTM standards.

Chemical Admixtures are added in accordance with the manufacture's recommendations. Texcrete reserves the right to adjust these dosages to meet changes in jobsite conditions and/or demands up to and including the substitution of equivalent products. Designed cementitious content is stated as a minimum. Texcrete reserves the right to increase cementitious content.

Aggregate weights may change depending on gradations or specific gravity of material. Mix Design Proportions and specifications are confidential and proprietary trade secrets of Texcrete. Any use or dissemination without permission is a violation of federal criminal law.

Will Squyres **Director of Technical Services**

Capitol Cement 11551 Nacogdoches Rd. San Antonio, Texas

> Date: August 9, 2021

Production Period: July 1, 2021
July 31, 2021 Beginning Ending

Type I (LA) Cement - C 150/C 150M

A Not Applicable
(D) Permisible to exceed this value provided expansion does not exceed 0.02% at 14 days. (C-1038) We certify that the above cement, at the time of shipment meets the chemical and physical requirements of

the current ASTM C 150 and AASHTO M 85 specifications.

The above data represents the averages of representative samples from production.

This product is made in the U.S.A

Signature

 $\Delta \sim 2$

Title

Chief Chemist

Douglas Conroy

CAPITOL AGGREGATES, INC., P.O.BOX 33240, SAN ANTONIO, TEXAS 78265-3240, PHONE: (210) 871-6100 WWW.CAPITOLAGGREGATES.COM

PO Box 38 Thompsons, TX 77481-0038 P: 281.343.0079

ASTM C618 / AASHTO M295 Testing of Class "F" Fly Ash Oak Grove Plant Franklin, Texas Unit #1,2

The test data listed herein was generated by applicable ASTM methods. The reported results pertain only to the
sample(s) or lot(s) tested. This report cannot be reproduced without permission from Boral Resources.

AUTHORIZED SIGNATURE:

Inchargement

Arcosa Aggregates 401 South Interstate Highway 45 Ferris, TX, 75125 T972.544.5900

Arcosa Aggregates Laboratory Testing

This material (Cameron-1336 / Plant #1) was sampled under on 05/03/21.

Summary of Results:

Decant = $0.3%$

#57 SPECS SHOWING

Sincerely, **Quality Control Department** Arcosa Aggregates 05/06/21

Arcosa Aggregates 401 South Interstate Highway 45 Ferris, TX, 75125 T972.544.5900

Arcosa Aggregates Laboratory Testing

This material (Cameron-1336) was sampled under ASTM D-75 on 06/02/21. The sample was reduced under ASTM C-702. This test was performed under ASTM C-136 & C-117.

Summary of Results:

Decant (-200) = 0.3% Unit Weight = 105.6 lbs pcf **Specific Gravity = 2.54** Absorption = 1.4%

Sincerely, **Quality Control Department** Arcosa Aggregates 06/04/21

Arcosa Aggregates 401 South Interstate Highway 45 Ferris, TX, 75125 T972.544.5900

Arcosa Aggregates Laboratory Testing

This material (Cameron-1336 / Plant #1) was sampled under on 06/02/21.

 $F.M. = 2.69$

Sand Equivalency = 88%

Load out SG-2.63 Unit Weight-101.4 Absorption-0.8% PLANT#1

Sincerely, **Quality Control Department** Arcosa Aggregates 06/04/21

HRWR - enhanced pumpability and strength development for HPC.

CHRYSO[®] Optima 249 is a new generation high range water reducing admixture based on proprietary polymers.

CHRYSO[®] Optima 249 is formulated specifically to enhance rheology and strengths characteristics of High and Ultra-high Performance Concrete (HPC & UHPC).

CHRYSO[®] Optima 249 exclusive formulation allows for extreme easiness of use and robustness.

CHRYSO[®] Optima 249 is manufactured under rigid quality control measures to provide uniform, reliable results.

Benefits

- · Provides enhanced workability retention
- . Provides increased slump and flowability without increased water content
- ·Improves finish, placement and pumpability of concrete
- Allows for ultra high strengths performances at all ages
- . Improves concrete quality by reducing the water-cement ratio for a given degree of workability
- . Proprietary molecule reduces concrete viscosity (stickiness) allowing for easier placement, better finish and enhanced pumpability
- Reduces cracking and shrinkage
- · Improves concrete chemical resistance and durability
- · Improves cementitious material performance (more psi/lb)

Areas of Application

CHRYSO[®] Optima 249 is recommended for all concrete mixes where significant water reduction, improved cementitious material performance (more psi/lb), improved finishing and enhanced slump retention characteristics are desirable including SCC.

CHRYSO[®] Optima 249 is especially recommended for use in high performance concrete applications where very good slump or flow retention characteristics and enhanced pumpability are required.

Standard PC HRWR CHRYSO Optima 249

www.chryso.com

CHRYSO ptima

Description:

Characteristics:

Physical state: liquid Color: Amber Density: Approx 1.07 pH: Approx. 4.5 CI ion content: Nil

CHRYSO® Optima 249 does not contain any purposely added calcium chloride or other chloride based components. It will not promote or contribute to corrosion of reinforcing steel in concrete.

Packaging:

55 gallon (210 L) drums 264 gallon (1000 L) totes bulk deliveries

Standard specifications:

CHRYSO® Optima 249 meets the requirements of ASTM C494, Types A & F for a high range water reducing admixture.

Directions for use:

Dosage

CHRYSO[®] Optima 249 is recommended for use at a dosage rate of 2 to 5 fluid ounces per 100 pounds (130 to 326 ml per 100 kg) of cementitious material for a Type A and 4 to 12. fluid ounces per 100 pounds (261 to 782 ml per 100 kg) of cementitious material for a Type F.

CHRYSO[®] Optima 249 can be added at the concrete plant or on the job site. In case of addition in a mixing truck, it is recommended that the concrete be mixed at high speed for 70 to 100 revolutions (approximately 4-6 minutes)

Because local job conditions vary, please contact your local Chryso sales representative for further assistance if using outside recommended dosage ranges.

CHRYSO Inc. Tel: (800) 936-7553 - Fax: 972-772-6010 Southern Division P.O. Box 190 Rockwall, TX 75032 Midwest Division P.O. Box 129 Charlestown, IN 47111 Western Division 5090 Nome St Denver, CO 80239

Directions for use:

Compatibility

CHRYSO[®] Optima 249 is compatible with all types of Portland cement, class C and F fly ash, slag, microsilica, calcium chloride, fibers and approved air entraining admixtures.

CHRYSO[®] Optima 249 can be used in all white, colored, and architectural concrete. For best results, each admixture must be dispensed separately into the concrete mix.

Precaution:

CHRYSO® Optima 249 may freeze at temperatures below 32° F (0° C). Although freezing does not harm CHRYSO® Optima 249 precautions should be taken to protect it from freezing.

If CHRYSO[®] Optima 249 should happen to freeze, thaw and reconstitute with mechanical agitation.

Do Not Use Pressurized Air For Agitation

Shelf Life: 9 months

\blacksquare Safety:

CHRYSO[®] Optima 249 is not considered dangerous to handle. Please refer to the material safety data sheet for additional information.

About CHRYSO:

CHRYSO is a worldwide leader for Concrete and Cement additives, CHRYSO has been servicing the construction Industry for over half a century with outstanding innovation and service. As a result, CHRYSO's name and products have been associated with the most prestigious and demanding construction projects worldwide.

The information contained in this document is given to the best of our knowledge and is the result of extensive and controlled testing. However, it cannot under any circumstances be
considered as a warrany involving our li

www.us.chryso.com/

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At Kiorized by Quality Assurance: مجمعها المستعمل
The results reported on this report represent the actual attributes of the material furnished and indicate full compliance with oil applicable specification and contract r

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Page: 1 of 1

09-22-2021 07:01 Load - 3937571

BL- 3906637

Heat - 1074611

bir466

Texas Corrugators, Inc.

Cust. PO - M-2921

Order - 20319253

L
RENNRKS : ALSO MEETS ASTM GRADE A36, A529-50, A529-55, A572-50, A572-55, A709-36, A709-50, AASHTO M270-38, M270-50, CSA G40.21-04 44W,50W, 55W
ASMIE SA-36 2008A ADDEND A

Page 1 OF 1 08/20/2021 14:54:27

TR No. 616221-01 45 2022-02-17

CERTIFILD MATERIAL TEST REPORT TO DIN EN 10204-2005 3.1

CONCRETE COMPRESSIVE STRENGTH TEST REPORT

Report Number: A1171057.0213 Service Date: 10/19/21 **Report Date:** 11/10/21 PO#616221-01 Task:

979-846-3767 Reg No: F-3272

Ambient Temp. (F):

Yield (Cu. Yds.):

Plastic Unit Wt. (pcf):

Comments: $F =$ Field Cured

Note: Reported air content does not include Aggregate Correction Factor (ACF).

Samples Made By: Terracon

Services: Obtain samples of fresh concrete at the placement locations (ASTM C 172), perform required field tests and cast, cure, and test compressive strength samples (ASTM C 31, C 39, C 1231). Start/Stop: 0900-1300

Terracon Rep.: Randy Rippstein Bill with TTI **Reported To:**

Contractor:

Report Distribution:
(1) Texas Transportation Institute, Gary Gerke (1) Terracon Consultants, Inc., Alex Dunigan, P.E.

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(1) Texas Transportation Institute, Bill Griffith

Test Methods: ASTM C 31, ASTM C143, ASTM C231, ASTM C1064

The tests were performed in general accordance with applicable ASTM, AASHTO, or DOT test methods. This report is exclusively for the use of the client
indicated above and shall not be reproduced except in full without the Page 1 of 2

CR0001, 11-16-12, Rev 6

CONCRETE COMPRESSIVE STRENGTH TEST REPORT

Report Number: A1171057.0213 Service Date: 10/19/21 **Report Date:** 11/10/21 Task: PO# 616221-01

Laboratory Test Data

Ambient Temp. (F):

Yield (Cu. Yds.):

Plastic Unit Wt. (pcf):

Comments: F = Field Cured

Note: Reported air content does not include Aggregate Correction Factor (ACF).

Samples Made By: Terracon

Services: Obtain samples of fresh concrete at the placement locations (ASTM C 172), perform required field tests and cast, cure, and test compressive strength samples (ASTM C 31, C 39, C 1231).

Terracon Rep.: Randy Rippstein Bill with TTI **Reported To:** Contractor:

Report Distribution:
(1) Texas Transportation Institute, Gary Gerke (1) Terracon Consultants, Inc., Alex Dunigan, P.E. (1) Texas Transportation Institute, Bill Griffith

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Reviewed By:

Start/Stop: 0900-1300

 ℓ l ℓ Alexander Dunigan

Project Manager

Test Methods: ASTM C 31, ASTM C143, ASTM C231, ASTM C1064

The tests were performed in general accordance with applicable ASTM, AASHTO, or DOT test methods. This report is exclusively for the use of the client
indicated above and shall not be reproduced except in full without the Page 2 of 2

CR0001, 11-16-12, Rev.6

TR No. 616221-01

CONCRETE COMPRESSIVE STRENGTH TEST REPORT

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145.2

Report Number: A1171057.0216 Service Date: 10/29/21 **Report Date:** 11/10/21 PO#616221-01 Task:

979-846-3767 Reg No: F-3272

Ambient Temp. (F): Plastic Unit Wt. (pcf): Yield (Cu. Yds.):

Concrete Temp. (F):

Comments: $F =$ **Field Cured**

Note: Reported air content does not include Aggregate Correction Factor (ACF).

Samples Made By: Terracon

Services: Obtain samples of fresh concrete at the placement locations (ASTM C 172), perform required field tests and cast, cure, and test compressive strength samples (ASTM C 31, C 39, C 1231). Start/Stop: 0900-1330

Terracon Rep.: Justin Maass **Reported To: Bill TAMU** Contractor:

Report Distribution:

(1) Texas Transportation Institute, Gary Gerke (1) Terracon Consultants, Inc., Alex Dunigan, P.E.

(1) Texas Transportation Institute, Bill Griffith

Test Methods: ASTM C 31, ASTM C143, ASTM C231, ASTM C1064

The tests were performed in general accordance with applicable ASTM, AASHTO, or DOT test methods. This report is exclusively for the use of the client
indicated above and shall not be reproduced except in full without the

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CONCRETE COMPRESSIVE STRENGTH TEST REPORT

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145.7

Report Number: A1171057.0216 Service Date: 10/29/21 **Report Date:** 11/10/21 Task: PO# 616221-01

Laboratory Test Data

Concrete Temp. (F):

Ambient Temp. (F):

Yield (Cu. Yds.):

Plastic Unit Wt. (pcf):

Comments: F = Field Cured

Note: Reported air content does not include Aggregate Correction Factor (ACF).

Samples Made By: Terracon

Services: Obtain samples of fresh concrete at the placement locations (ASTM C 172), perform required field tests and cast, cure, and test compressive strength samples (ASTM C 31, C 39, C 1231).

Terracon Rep.: Justin Maass **Reported To: Bill TAMU** Contractor:

Report Distribution:

(1) Texas Transportation Institute, Gary Gerke (1) Terracon Consultants, Inc., Alex Dunigan, P.E. (1) Texas Transportation Institute, Bill Griffith

Reviewed By:

Start/Stop: 0900-1330 ℓ l ℓ Alexander Dunigan

Project Manager

Test Methods: ASTM C 31, ASTM C143, ASTM C231, ASTM C1064

The tests were performed in general accordance with applicable ASTM, AASHTO, or DOT test methods. This report is exclusively for the use of the client
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APPENDIX C. MASH TEST 4-12(CRASH TEST NO. 616221-01)

C.1 VEHICLE PROPERTIES AND INFORMATION

Table C.1. Vehicle Properties for Test No. 616221-01.

Table C.1. Vehicle Properties for Test No. 616221-01. (Continued)

Other notes to include ballast type, dimensions, mass, location, center of mass, and method of attachment:

TWO BLOCKS H 30 W 60 L 30

CENTERED IN MIDDLE OF BED

TIED DOWN WITH FOUR 3/8 CABLES PER BLOCK

64.75 FROM GROUND TO CENTER OF BLOCK

Table C.2. Exterior and Occupant Compartment Measurements for Test No. 616221-01.

2021-11-8 Test No.: 616221-01 VIN No.: 1FVACXCS14HM27927

Date:

C.2. SEQUENTIAL PHOTOGRAPHS

 $0.300 s$ **Figure C.1. Sequential Photographs for Test No. 616221-01 (Overhead and Frontal Views).**

Figure C.1. Sequential Photographs for Test No. 616221-01 (Overhead and Frontal Views) (Continued).

 $0.000 s$ 0.400 s

 0.300 s 0.700 s **Figure C.2. Sequential Photographs for Test No. 616221-01 (Rear View).**

C.3. VEHICLE ANGULAR DISPLACEMENTS

Roll, Pitch and Yaw Angles

Figure C.3. Vehicle Angular Displacements for Test No. 616221-01.

C.4. VEHICLE ACCELERATIONS

Figure C.4. Vehicle Longitudinal Accelerometer Trace for Test No. 616221-01 (Accelerometer Located at Center of Gravity).

Figure C.5. Vehicle Lateral Accelerometer Trace for Test No. 616221-01 (Accelerometer Located at Center of Gravity).

Figure C.6. Vehicle Vertical Accelerometer Trace for Test No. 616221-01 (Accelerometer Located at Center of Gravity).